STRUCTURAL GENERAL NOTES
DIVISION 1 – GENERAL REQUIREMENTS

1. THESE DRAWINGS INCLUDING THE INFORMATION CONTAINED HEREON SHALL REMAIN THE PROPERTY OF FALL RIVER HOLDINGS, LLC., WORLDWIDE STEEL BUILDINGS, AND VAN DEURZEN AND ASSOCIATES, P.A. AND ARE PROVIDED SOLELY FOR ERECTING THE BUILDING DESCRIBED IN THE APPLICABLE PURCHASE ORDER AND SHALL NOT BE MODIFIED, REPRODUCED OR USED FOR ANY OTHER PURPOSE WITHOUT PRIOR WRITTEN CONSENT OF FALL RIVER HOLDINGS, LLC., WORLDWIDE STEEL BUILDINGS, AND VAN DEURZEN AND ASSOCIATES, P.A.
2. THE GENERAL CONTRACTOR AND/OR ERECTOR SHALL BE SOLELY RESPONSIBLE FOR GOOD QUALITY WORKMANSHIP IN ERECTING THIS BUILDING IN CONFORMANCE WITH THESE DRAWINGS, DETAILS REFERENCED ON THESE DRAWINGS, WORLDWIDE STEEL BUILDING ERECTION MANUAL AND INDUSTRY STANDARDS PERTAINING TO PROPER ERECTION, INCLUDING THE CORRECT USE OF TEMPORARY BRACING.
3. THE CONTRACTOR AND/OR ERECTOR SHALL EXAMINE ACTUAL JOB CONDITIONS AND BE RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND ELEVATIONS SHOWN ON THESE ERECTION PLANS WITH THOSE SHOWN ON THE CONTRACT DOCUMENTS. IF ERRORS, OMISSIONS OR DISCREPANCIES ARE FOUND THEY SHALL BE REPORTED TO VAN DEURZEN AND ASSOCIATES, P.A. BEFORE PROCEEDING WITH THE WORK.
4. INSTALLATION OF ADEQUATE TEMPORARY BRACING FOR USE UNTIL CONSTRUCTION IS COMPLETE IS THE RESPONSIBILITY OF THE ERECTOR.
5. EXACT LOCATION OF WINDOWS, OVERHEAD DOORS, AND WALK DOORS SHALL BE DETERMINED DURING BUILDING ERECTION. TRUSSES AND DIAGONAL BRACING SHALL NOT BE MOVED TO INSTALL WINDOWS, OVERHEAD DOORS, AND WALK DOORS.
6. CONCRETE GROUT, ANCHOR BOLTS, AND ANY OTHER EMBEDDED ITEMS ARE TO BE FURNISHED BY OTHERS.
7. ANCHOR BOLT DIAMETERS WERE DETERMINED BY ALLOWABLE SHEAR AND TENSION PER AISC SPECIFICATIONS (FY = 36 KSI). ANCHOR BOLT LENGTH AND METHOD OF TRANSFERRING FORCES FROM ANCHOR BOLTS TO FOOTINGS ARE TO BE DETERMINED BY OTHERS.
8. DESIGN LOADS AND REACTIONS ARE FURNISHED ON THIS DRAWING.
9. FOUNDATION MUST BE LEVEL, SQUARE, AND SMOOTH. ANCHOR BOLTS MUST BE ACCURATELY PLACED AS SHOWN ON THIS DRAWING OR STEEL WILL NOT FIT.
10. ALL BOLTS TO BE SAE J429 GRADE 5 – BOLTS SHALL BE TIGHTENED IN ACCORDANCE WITH THE "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS" SUBSECTION TITLED "TURN-OF-NUT TIGHTENING".
11. STRUCTURAL STEEL ANGLES AND WEB MEMBERS RODS ARE ASTM A572 GRADE 50, SIZES ARE NOTED ON SHOP DRAWINGS. SHOP DRAWINGS ARE NOT PROVIDED WITH THE ERECTION SET UNLESS REQUESTED.
12. WIND RODS ARE 1/2"ø ASTM A572 GRADE 50 (UNLESS NOTED OTHERWISE) WITH ENDS THREADED, DOUBLE WHERE NOTED ON PLANS/ELEVATIONS
13. MISCELLANEOUS PLATE ARE ASTM A36
14. WIND AND TRUSS CHORD BRACING ARE AN INTEGRAL PART OF THE STRUCTURAL SYSTEM AND MUST BE PROPERLY INSTALLED PRIOR TO ERECTION OF WALL AND ROOF SHEETS. REMOVAL OR ALTERATION OF WALL BRACING AND TRUSS CHORD BRACING WITHOUT PRIOR AUTHORIZATION IS PROHIBITED.
15. BUILDING SUBFRAMING:

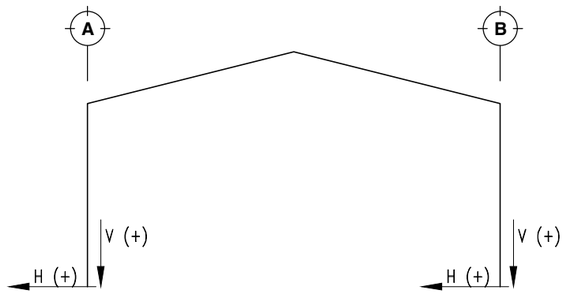
    ALL LIGHT GAUGE FRAMING SHALL CONFORM TO THE REQUIREMENTS OF ASTM A653 GRADE 50 WITH A MINIMUM YIELD STRESS OF 50000 PSI.

    a. GIRTS: 3 5/8" x 1 5/8" x 20ga
    b. PURLINS: 6" x 1 5/8" x 20ga

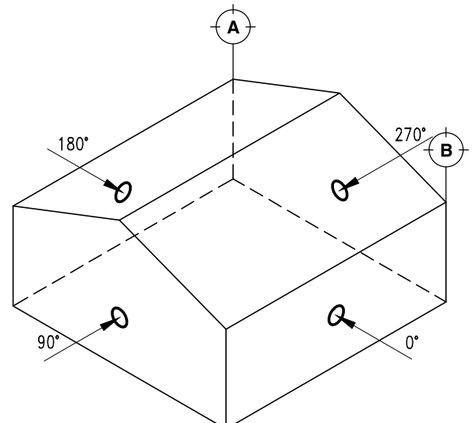
BUILDING CODE	2012 IBC/ASCE 7-10
DESIGN LOADS	
ROOF LIVE	20 PSF Reduced Per ASCE 7-10 Sec 4.8
ROOF DEAD (COLLATERAL)	10 PSF (2 PSF + SELF WEIGHT + 5 PSF SOLAR)
ROOF SNOW LOAD	30 PSF
GROUND SNOW LOAD	43 PSF (UNHEATED)
WIND	
BASIC WIND SPEED	115 MPH (Vult)
EXPOSURE	C
RISK CATEGORY	II

	COLUMN GRID A		COLUMN GRID B	
	HORIZONTAL REACTION (lbs)	VERTICAL REACTION (lbs)	HORIZONTAL REACTION (lbs)	VERTICAL REACTION (lbs)
DEAD LOAD	70	2723	-70	2723
ROOF LIVE LOAD	57	3532	-57	3532
SNOW LOAD	-26	6338	26	6338
Wind 0° - Neg (MWFRS Load Case A)	-217	-4270	-903	-1987
Wind 0° - Pos (MWFRS Load Case A)	-699	-2320	-594	-1381
Wind 180° - Neg (MWFRS Load Case A)	861	-1857	240	-4287
Wind 180° - Pos (MWFRS Load Case A)	552	-1250	721	-2337
Wind 90° - Neg (MWFRS Load Case B)	783	-2907	-465	-1936
Wind 90° - Pos (MWFRS Load Case B)	679	-2146	-362	-1175
Wind 270° - Neg (MWFRS Load Case B)	465	-1936	-783	-2907
Wind 270° - Pos (MWFRS Load Case B)	362	-1175	-679	-2146
LIVE LOAD (ADDITIONAL)	0	0	0	0

Notes:
1. ALL REACTIONS ARE UNFACTORED
2. NO REACTIONS PROVIDED FOR ENDWALL COLUMNS, ENDWALLS ARE DESIGNED AS SLIP CONNECTIONS (NO GRAVITY LOADS) UNLESS NOTED OTHERWISE. HORIZONTAL REACTIONS FOR WIND LOADS ARE LESS THAN OR EQUAL TO MAIN FRAME COLUMNS
3. ALL FRAMES (INCLUDED TERMINAL BAYS) ARE DESIGNED FOR THE SAME LOADS UNLESS NOTED OTHERWISE. BUILDING IS DESIGNED TO BE EXPANDED IN THE FUTURE, UNLESS THE OWNER SPECIFICALLY REQUESTS AN ALTERNATE DESIGN; IN THAT CASE, LOADS CAN BE HALVED



LOADING DIAGRAM



WIND DIAGRAM (ASCE MWFRS 28.4-1)

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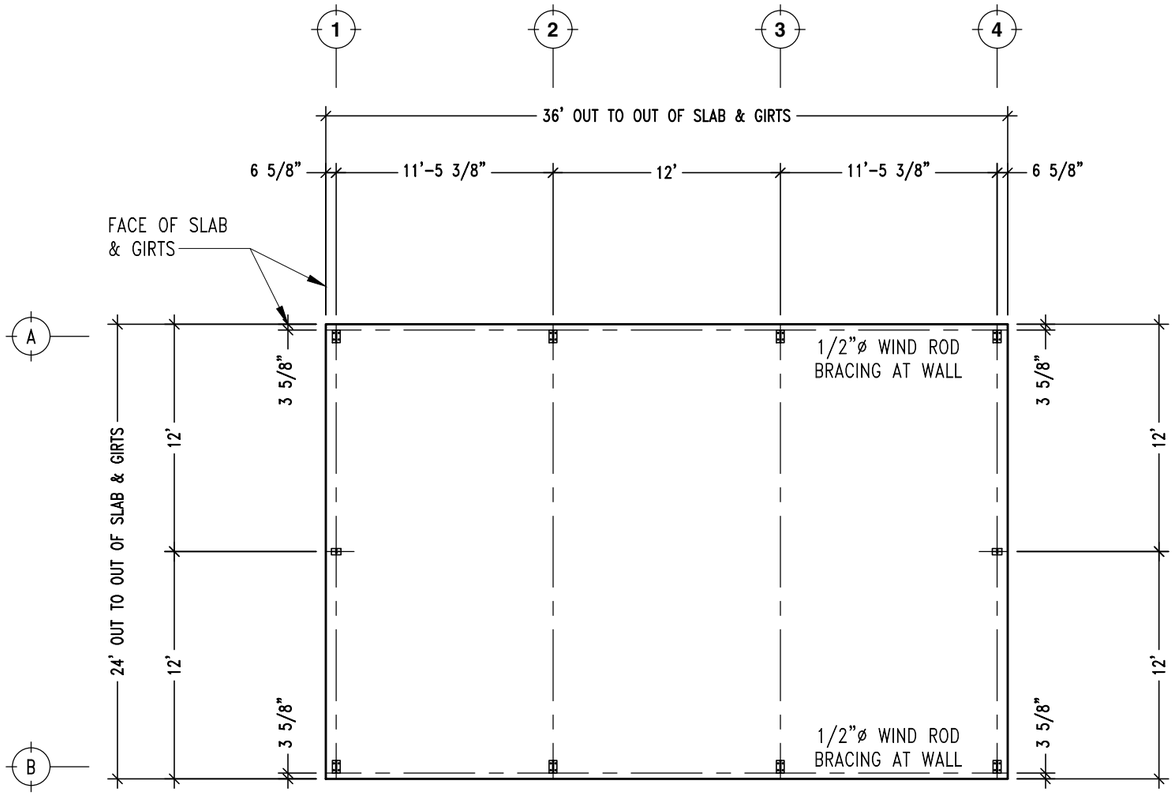
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WILDCAT GLADES FRIENDS GROUP
(WW JOB #59-60)
201 W RIVIERA

LOCATION: JOPLIN, MO

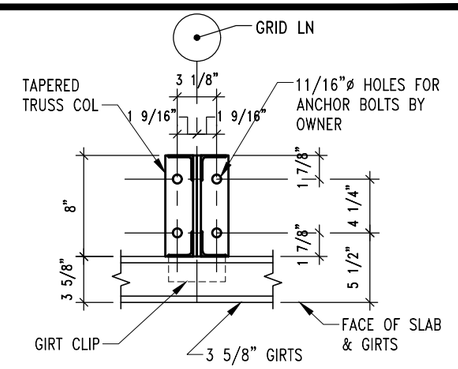
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DATE:	11/22/2019
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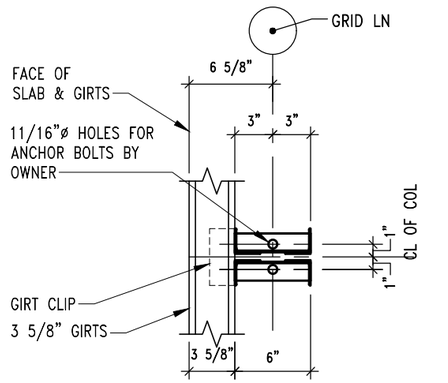
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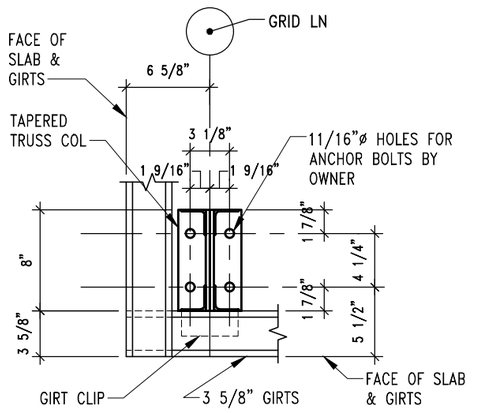
ANCHOR BOLT SETTING PLAN
SCALE: 1/8" = 1'-0"



SIDEWALL ANCHOR BOLT
SCALE: 1"= 1'-0"



ENDWALL ANCHOR BOLT
SCALE: 1"= 1'-0"



CORNER ANCHOR BOLT
SCALE: 1"= 1'-0"

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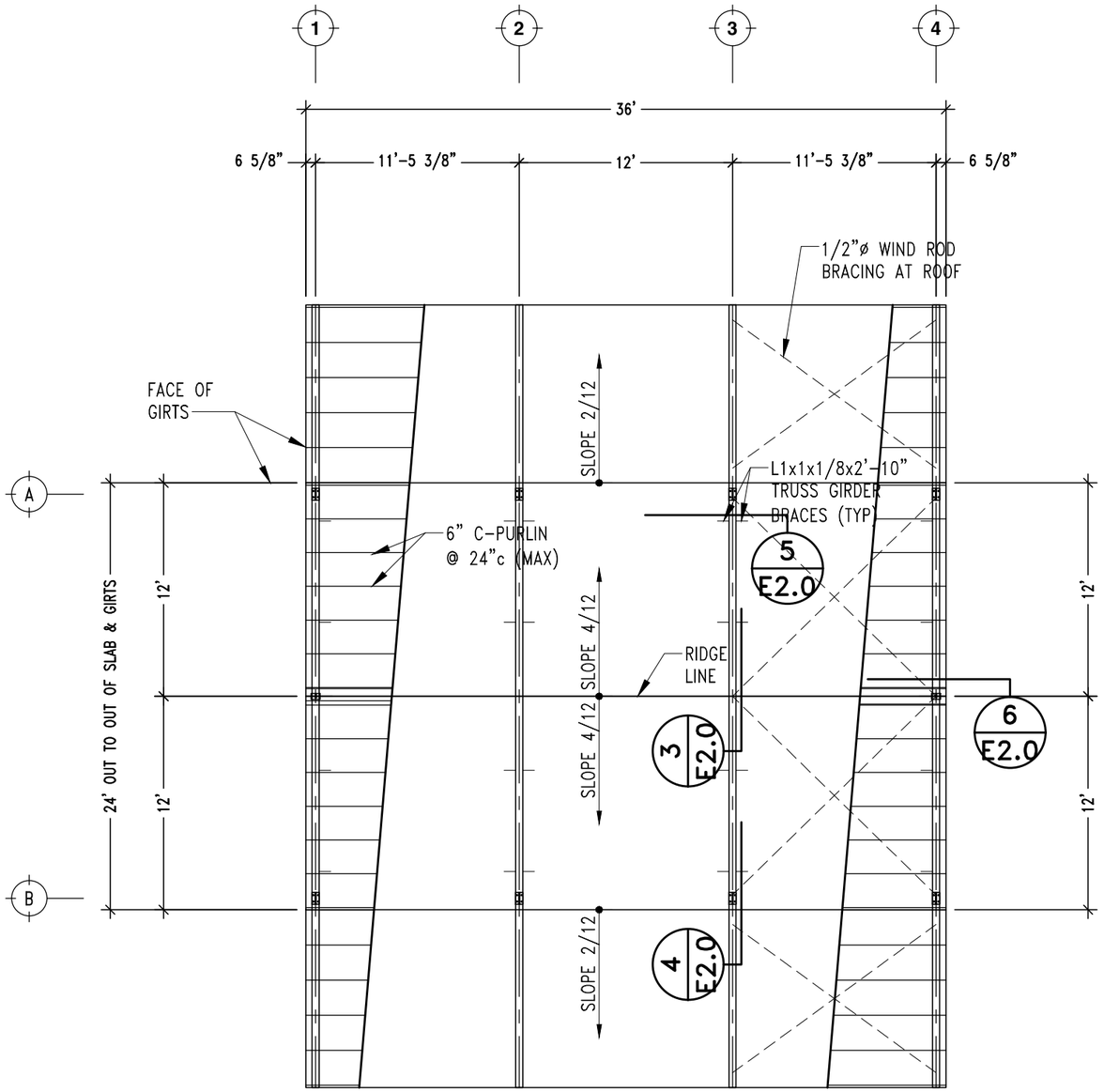
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WILDCAT GLADES FRIENDS GROUP
(WW JOB #59-60)
201 W RIVIERA

LOCATION: JOPLIN, MO

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ROOF FRAMING PLAN
SCALE: 1/8" = 1'-0"

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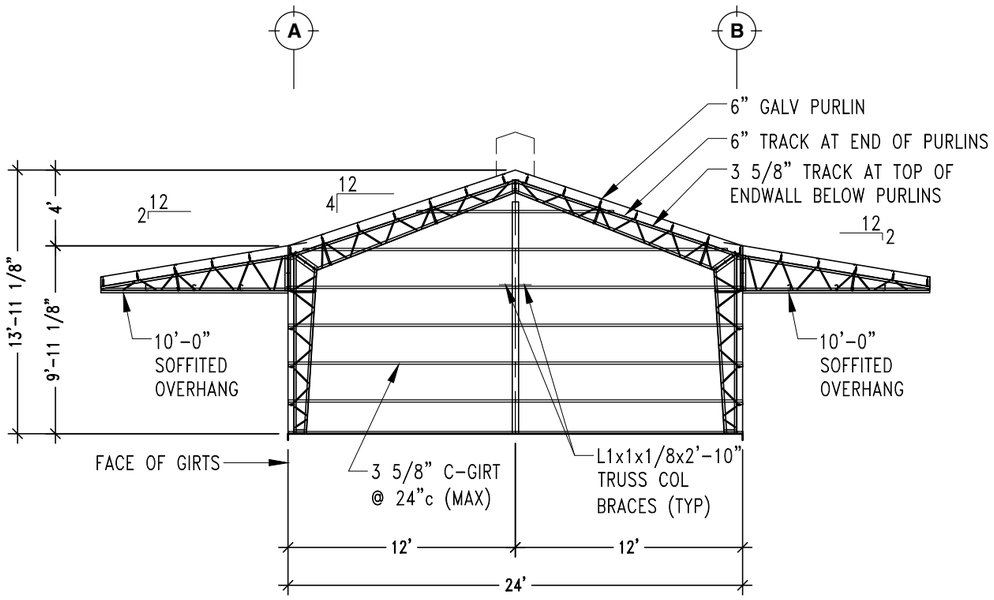
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201 W RIVIERA

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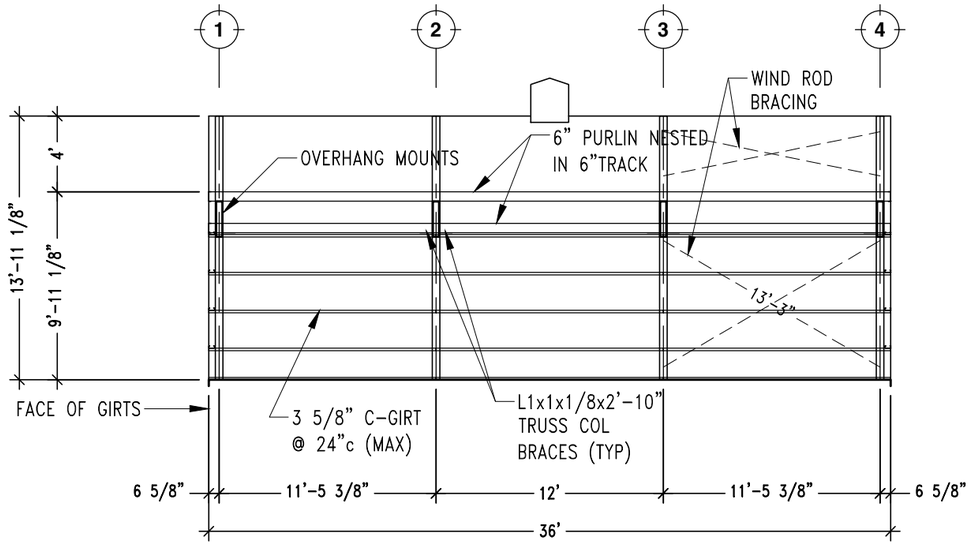
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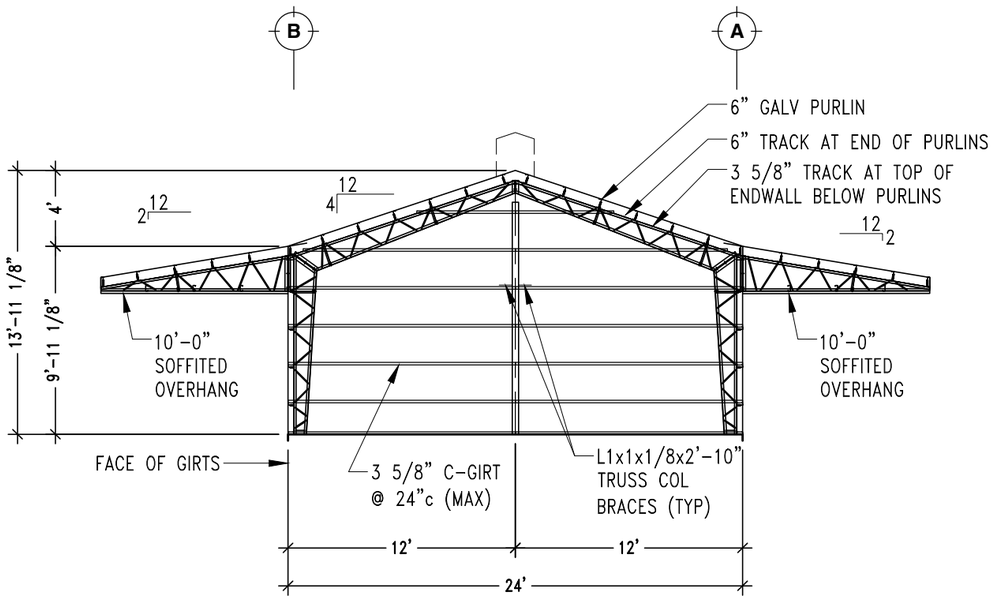
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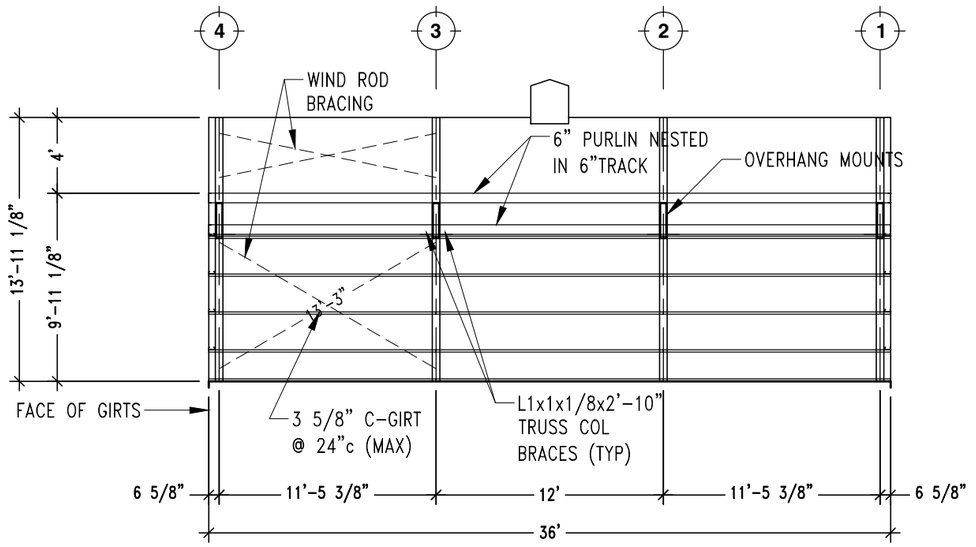
FRONT ELEVATION
SCALE: 1/8" = 1'-0"



RIGHT ELEVATION
SCALE: 1/8" = 1'-0"



REAR ELEVATION
SCALE: 1/8" = 1'-0"



LEFT ELEVATION
SCALE: 1/8" = 1'-0"

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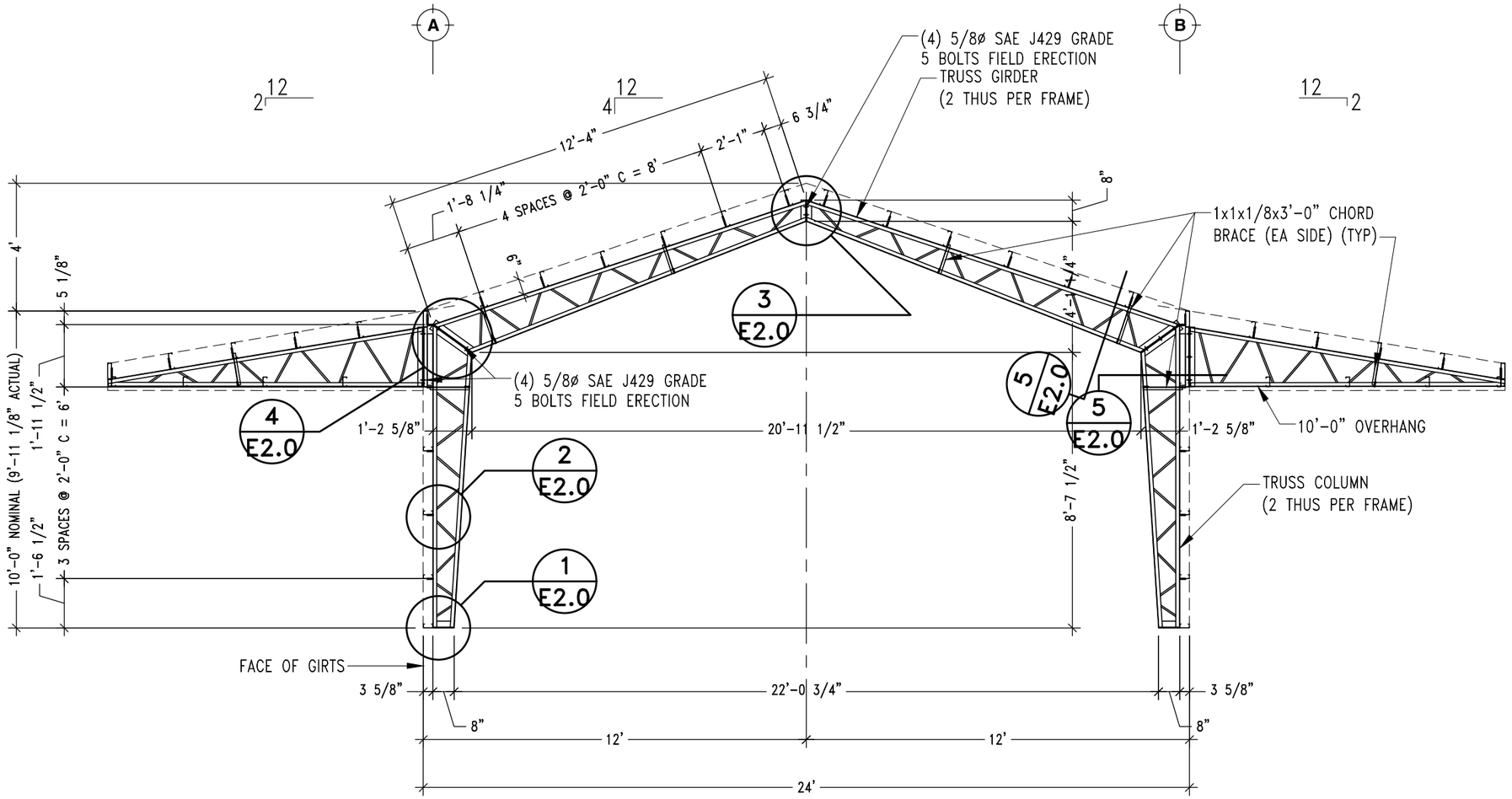
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TYPICAL BUILDING SECTION

SCALE: 1/4" = 1'-0"

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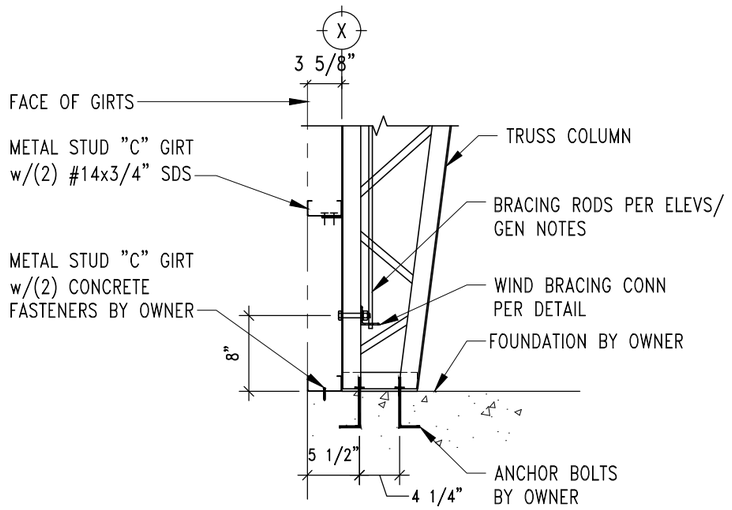
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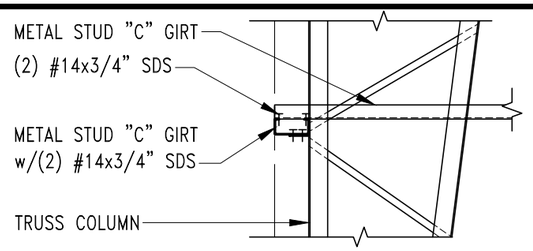
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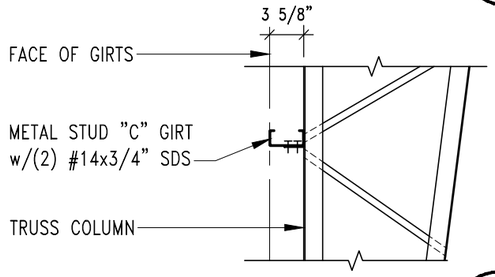
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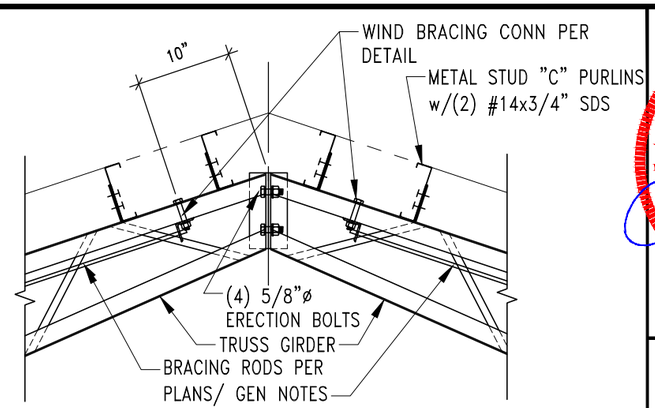
COLUMN BASE DETAIL — 1 / E2.0
SCALE: 3/4" = 1'-0"



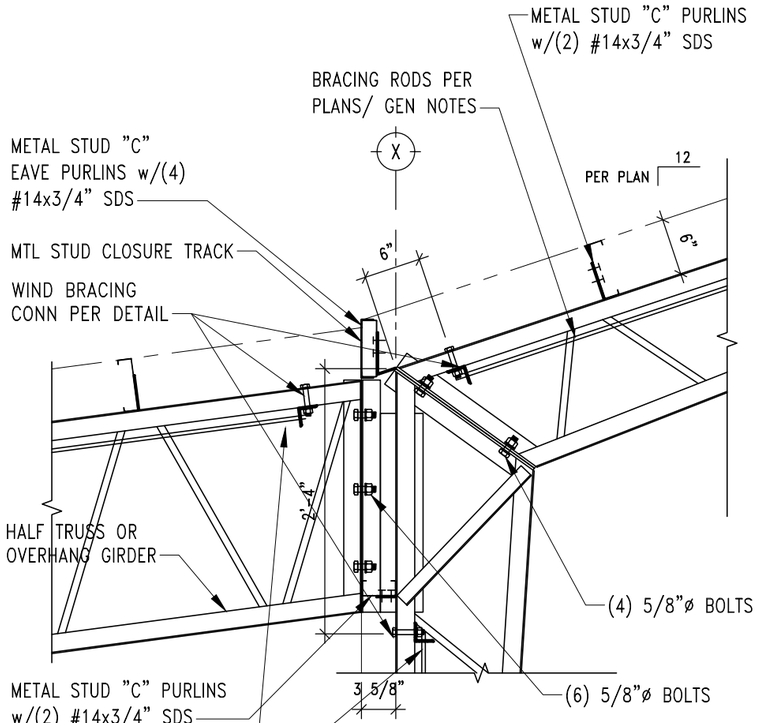
GIRT CORNER CONN. — 2a / E2.0
SCALE: 3/4" = 1'-0"



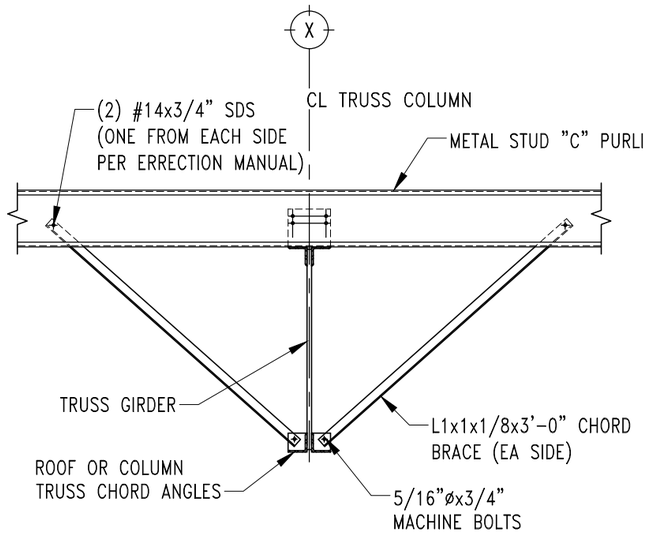
GIRT CONN. DETAIL — 2 / E2.0
SCALE: 3/4" = 1'-0"



RIDGE DETAIL — 3 / E2.0
SCALE: 3/4" = 1'-0"

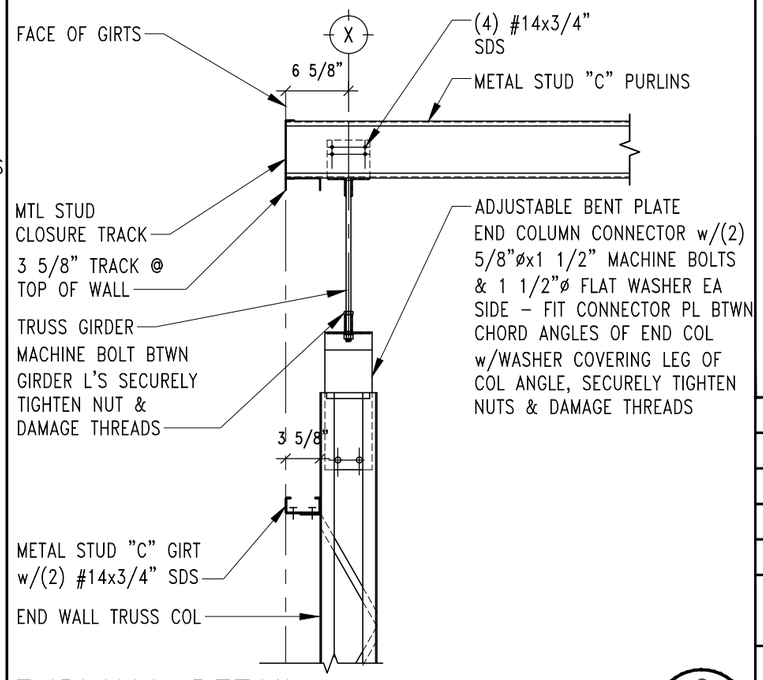


KNEE DETAIL — 4 / E2.0
SCALE: 3/4" = 1'-0"



NOTE: WHERE BRACES ARE CROWDED BY A DOOR, FIELD CUT THE ANGLE (& DRILL HOLE) TO PLACE THE END AS CLOSE TO THE DOOR JAMB AS POSSIBLE

TRUSS CHORD BRACE — 5 / E2.0
SCALE: 3/4" = 1'-0"



ENDWALL DETAIL — 6 / E2.0
SCALE: 3/4" = 1'-0"

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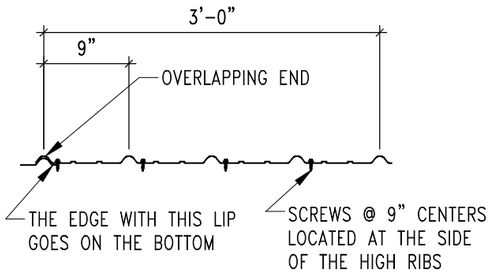
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201 W RIVIERA

LOCATION: JOPLIN, MO

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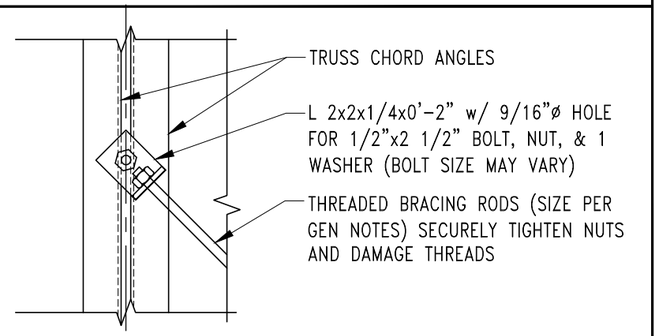
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E2.0	⚠0



29 GAUGE (80 KSI) RIBBED PANEL at ROOF AND WALLS – ATTACH WITH #12 x 1 1/4" METAL TO METAL SELF DRILLING SCREWS WITH RUBBER WASHERS
ALTERNATE: IF USING WOOD SUBFRAMING ATTACH WITH #9 x 1 1/2" WOOD SCREWS

WALL / ROOF PANEL ATTACHMENT

SCALE: 3/4"= 1'-0"



WIND BRACING CONNECTION (SINGLE)

SCALE: 2"= 1'-0"

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E2.1	0